DEVELOPMENT PARAMETERS

PLOT RATIO : 0.45 (ABOUT) SITE COVERAGE : 34 % (ABOUT)

NO. OF STRUCTURE ::

BUILDING HEIGHT : 3 - 8 m (ABOU' NO. OF STOREY : 1 - 2

PARKING AND LOADING/UNLOADING PROVISIONS

NO. OF PRIVATE CAR PARKING SPACE : 1

DIMENSION OF PARKING SPACE : 5m (L) X 2.5m (W)

NO. OF L/UL SPACE FOR LIGHT GOODS VEHICLE

DIMENSION OF L/UL SPACE : 7m (L) X 3.5m (W)

FIRE SERVICE INSTALLATIONS

ρ٩

EMERGENCY LIGHT

EXIT

EXIT SIGN

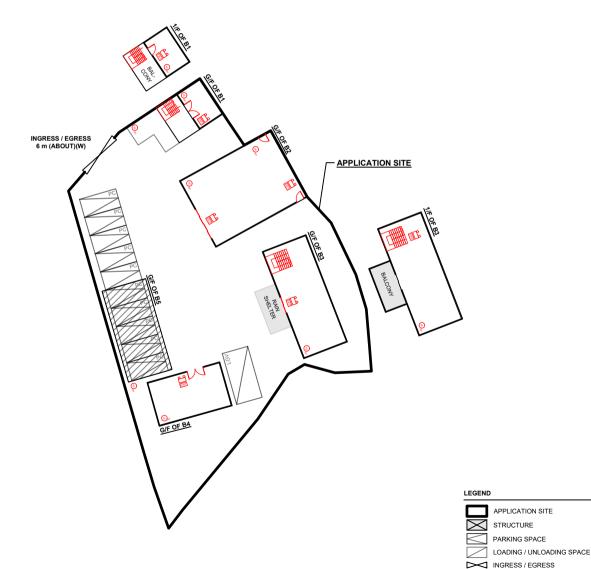
(FE)_{DP}

4 KG DRY POWDER TYPE FIRE EXTINGUISHER

FS NOTES:

- SUFFICIENT EMERGENCY LIGHTING SHALL BE PROVIDED THROUGHOUT THE ENTIRE BUILDING IN ACCORDANCE WITH BS5266-1:2016 AND BS EN1838:2013 AND FSD CIRCULAR LETTER 6/2021
- SUFFICIENT DIRECTIONAL AND EXIT SIGN SHALL BE PROVIDED IN ACCORDANCE WITH BS5266: PART 1 AND FSD CIRCULAR LETTER 5/2008.
- PORTABLE HAND-OPERATED APPROVED APPLIANCE SHALL BE PROVIDED AS REQUIRED BY OCCUPANCY.
- 4. ACCESS IS PROVIDED FOR EMERGENCY VEHICLE TO REACH 30m OF ALL PART OF STRUCTURES.

STR	UCTURE	USE	COVERED AREA	GFA	BUILDING HEIGHT
B1	(G/F)	RESTAURANT AND OUTSIDE SEATING ACCOMMODATION OF RESTAURANT	60 m ² (ABOUT)	60 m ² (ABOUT)	7 m (ABOUT)(2-STOREY)
	(1/F)	RESTAURANT AND WASHROOM		40 m ² (ABOUT)	
B2		RESTAURANT, KITCHEN & WASHROOM	138 m ² (ABOUT)	138 m ² (ABOUT)	3.5m (ABOUT)(1-STOREY)
В3	(G/F) (1/F)	SHOP AND SERVICES OFFICE AND WASHROOM	108 m ² (ABOUT)	108 m ² (ABOUT) 108 m ² (ABOUT)	8m (ABOUT)(2-STOREY)
В4		SHOP AND SERVICES	60 m ² (ABOUT)	60 m ² (ABOUT)	3m (ABOUT)(1-STOREY)
B5		RAIN SHELTER FOR PARKING SPACE	75 m² (ABOUT)	75 m² (ABOUT)	3m (ABOUT)(1-STOREY)
		TOTAL	441 m² (ABOUT)	589 m² (ABOUT)	





PLANNING CONSULTANT



PROJEC

PROPOSED TEMPORARY SHOP AND SERVICES AND EATING PLACE WITH ANCILLARY FACILITIES FOR A PERIOD OF 5 YEARS

SITE LOCATIO

LOT 390 RP (PART) IN D.D. 106, KAM SHEUNG ROAD, YUEN LONG, NEW TERRITORIES

SCALE	
1:500 @ A4	
DRAWN BY	DATE
MN	12.9.2024
REVISED BY	DATE
APPROVED BY	DATE

DWG. TITLE FSIs PROPOSAL

DWG NO. VER.
APPENDIX I 001

Temporary Shop and Services and Eating Place for a Period of 5 Years in "Village Type Development" zone, Lot 390 RP (Part) in D.D. 106, Kam Sheung Road, Yuen Long, New Territories

Drainage Appraisal

September 2024

Draina	ge An	nraisal
Diania	5C / 1P	, pi aisai

Ta				•			
12	n	ΙΔ	∩t		nη	ΤΔ	nt
10	u		VI.	_		ııc	

1.	Intro	oduction	1
		Background	
		The Site	
		elopment Proposal	
		The Proposed Development	
		essment Criteria	
		ting and Proposed Drainage System	
		clusion	
Э.	COII	CIUSIOII	_

List of Table

Table 1 - Key Development Parameters	2
Table 2– Design Return Periods under SDM	2

List of Figure

Figure 1 – Site Location Plan

Figure 2 - Existing Drainage Plan

Figure 3 – Existing Internal Drainage System

Figure 4 – Catchment Plan

List of Appendix

Appendix A – Existing U Channel Design Checking

Appendix B - Development Layout Plan

Appendix C – Reference Drawings for UChannel and Catchpit

Drainage Appraisal

1. Introduction

1.1 Background

- 1.1.1 The applicant seeks to use Lot 390 RP (Part) in D.D. 106, Kam Sheung Road, Yuen Long, New Territories (the Site) for 'Temporary Shop and Services and Eating Place for a Period of 5 Years (Proposed Development).
- 1.1.2 This Drainage Proposal is to support the planning application for the proposed use.

1.2 The Site

- 1.2.1 The major portion of the area was approved site under A/YL-KTS/990. The Application Site area is about 1,298m², and it situates beside Kam Sheung Road to the west. The site is currently paved and occupied by existing structures. The site location plan is shown in **Figure 1**.
- 1.2.2 The Application Site is surrounded by temporary structures. It is generally flat with existing ground level of approx. +9.8 mPD.
- 1.2.3 There is an existing public drainage and ditch along Kam Sheung Road. Existing Drainage Plan is shown in **Figure 2** for reference. There are existing internal drainage channels within site area, the existing internal drainage system is shown in **Figure 3**.
- 1.2.4 Proposed Development Layout plan is shown in **Appendix B** for reference.

Page | 1 Sep-24

2. Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 1,298m². The indicative development schedule is summarized in **Table 1** below for technical assessment purpose.

Proposed Development	
Total Site Area (m ²)	1,298
Assume all proposed site area as paved	1,298
area after development for assessment	
purpose (m ²)	

Table 1 - Key Development Parameters

3. Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this DIA. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2- Design Return Periods under SDM

3.1.2 The site and the surroundings are generally flat. The proposed village drainage system intended to collect runoff from the internal site and discharge to existing nearby public drainage system. 1 in 10 years return period is adopted for the drainage design.

Page | 2 Sep-24

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Headquarters Rainfall Zone. Therefore, for 10 years return period, the following values are adopted.

(up to Corrigendum No.1/2024)

$$a = 485$$
 $b = 3.11$
 $c = 0.397$

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278CiA$

where
$$Q_p$$
 = peak runoff in m³/s
 C = runoff coefficient (dimensionless)
 i = rainfall intensity in mm/hr
 A = catchment area in km²

3. The run-off coefficient (C) of surface runoff are taken as follows:

Paved Area: C = 0.95
 Unpaved Area: C = 0.35

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the flow (m/s)

S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:
$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V = velocity of the pipe flow (m/s)

 S_f = hydraulic gradient

 k_f = roughness value (m)

v = kinematics viscosity of fluid

D = pipe diameter (m)

R = hydraulic radius (m)

4. Existing and Proposed Drainage System

- 4.1.1 The Application Site and the surrounding areas are generally flat. There are existing internal Uchannels and catchpit within Application Site. Design review on existing Uchannels has been conducted. Most of the existing channels are capable of carrying the runoff from the Application Site. Two of the channels are proposed to be updated. The alignment, size and gradient of the existing internal drains and proposed channels are shown in **Figure 3**. The catchment plan is shown in **Figure 4**. The checking of channels are shown in **Appendix A**.
- 4.1.2 There is no change in drainage characteristics of the site, it is anticipated that there will be no significant drainage impact to the area.
- 4.1.3 The reference standard drawings of existing drains are shown in **Appendix C**.

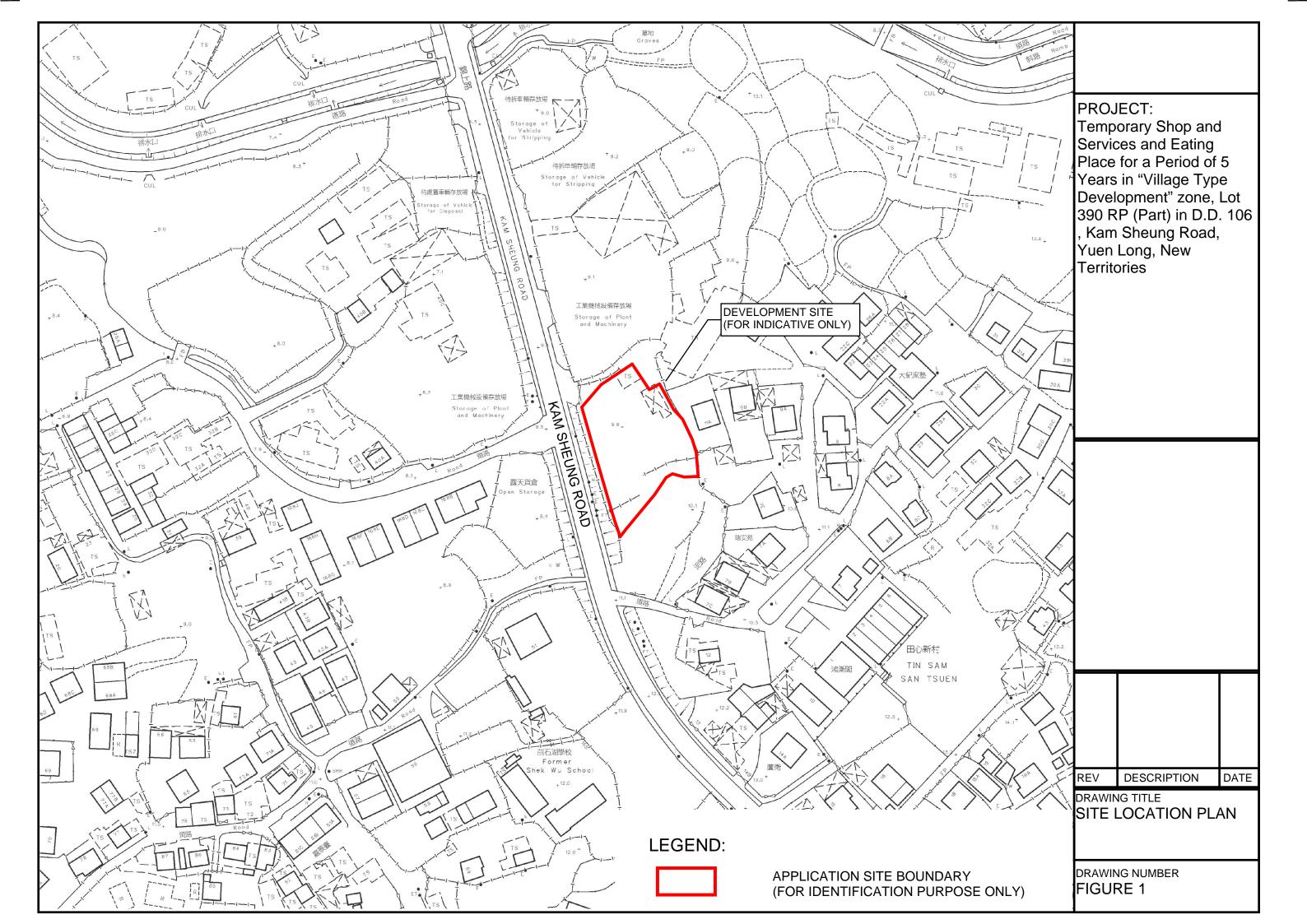
5. Conclusion

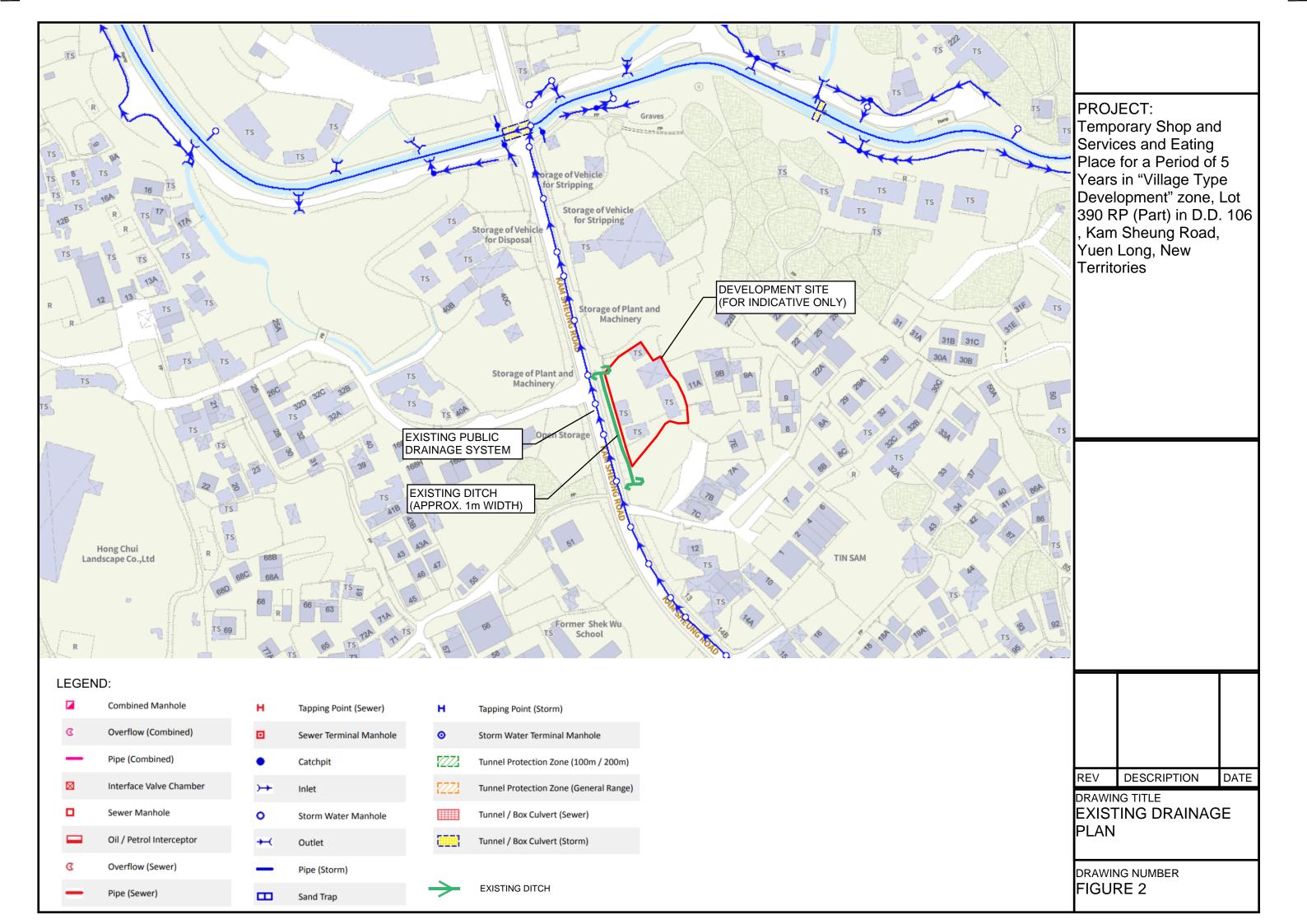
- 5.1.1 A drainage appraisal has been conducted for the Proposed Development. The surface runoff from the Application Site will be collected by the existing/proposed internal Uchannels and discharged to the existing ditch/ drainage system beside Kam Sheung Road.
- 5.1.2 With the proposed drainage system, it is anticipated that there will be no significant drainage impact to the area after the implementation of the development.

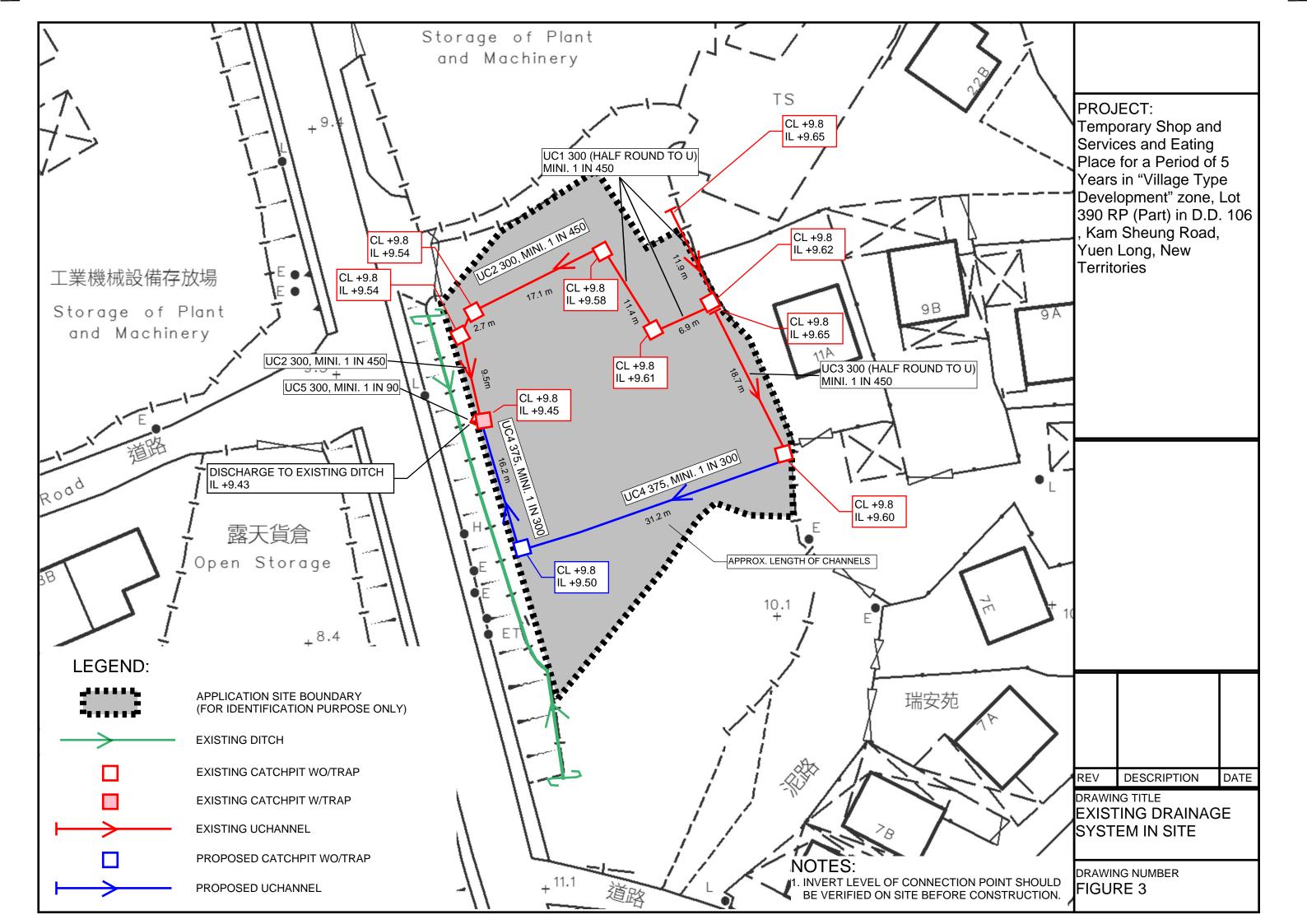
- End of Text -

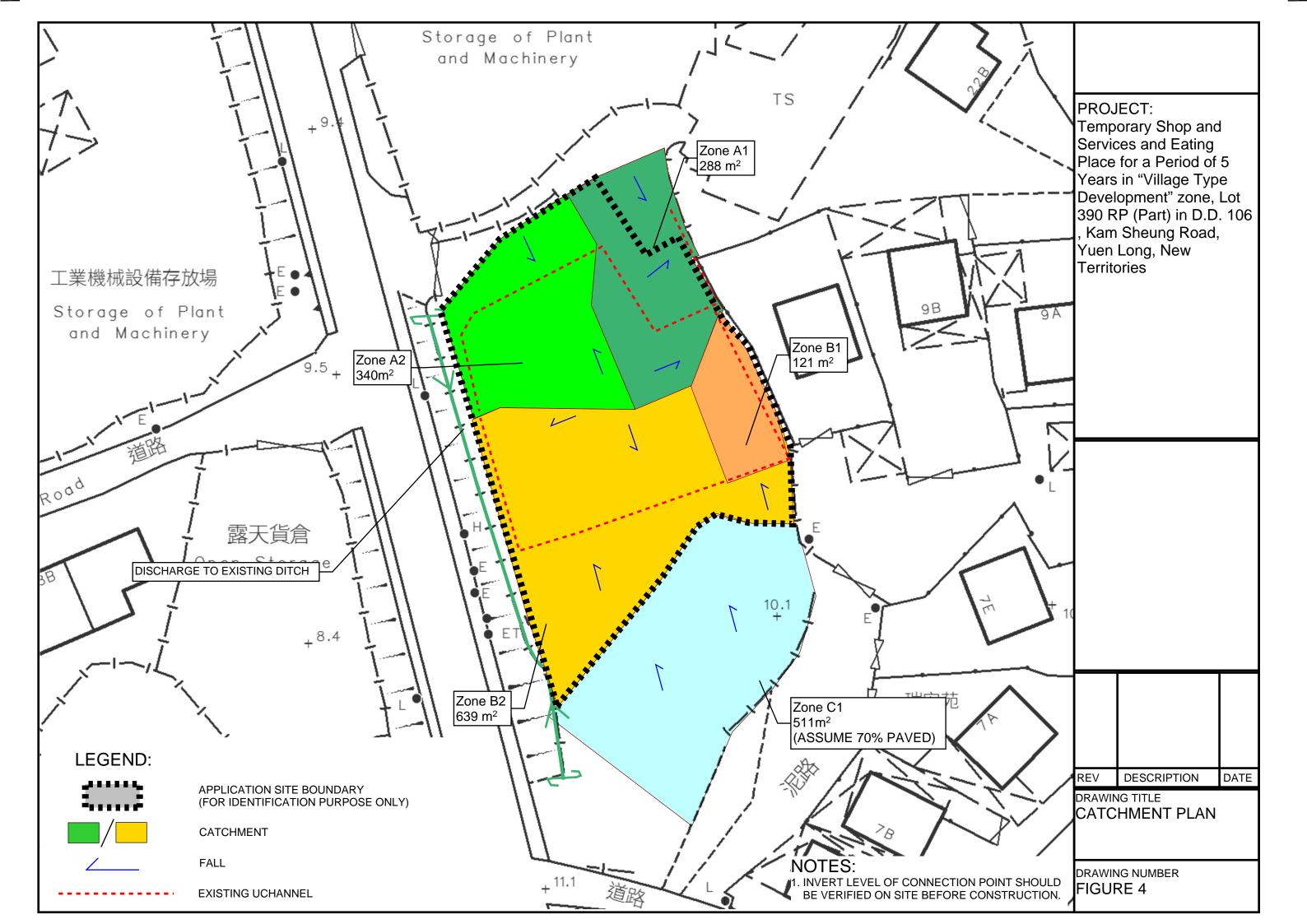
Page | 5

FIGURES









Appendix

Appendix A - Existing U Channel Design Checking

U Channel 1 (Zone A1)

Runoff Estimation				
Design Return Period		1 in	10	years
Paved Area	288 =		288	(m2)
Unpaved Area			0	(m2)
Total Equivalent Area	288 x 0.95 + 0 x 0.35 =		274	(m2)
Time of Concentration			2.68	min
Rainfall Intensity, I *			242	mm/hr
Design Discharge Rate, Q	0.278 x 274 x 242 / 1000000 =		0.018	m3/s

$$_{\star} \quad i = \frac{a}{(t_d + b)^c}$$

a=485, b=3.11, c=0.397

Channel Size		300	(mm)
Gradient, Sf	1 in	450	
Area (Half Round)		0.035	(m2)
Wetted Perimeter		0.471	(m)
R		0.075	(m)
Velocity**		0.60	m/s
Capacity		0.021	m3/s

**
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Utilization 0.018 / 0.021 = **86.80** % OK (allowed for 10% for siltation)

U Channel 2 (Zone A1 + A2)

Runoff Estimation				
Design Return Period		1 in	10	years
Paved Area	629 =		629	(m2)
Unpaved Area			0	(m2)
Time of Concentration			2.68	min
Total Equivalent Area	$629 \times 0.95 + 0 \times 0.35 =$		598	(m2)
Rainfall Intensity, I *			242	mm/hr
Design Discharge Rate, Q	0.278 x 0 x 242 / 1000000 =		0.040	m3/s

$$i = \frac{a}{(t_d + b)^c}$$

U Channel			
Channel Size		300	(mm)
Gradient	1 in	450	
Area		0.080	(m2)
Wetted Perimeter		0.771	(m)
R		0.104	(m)
Velocity**		0.75	m/s
Capacity		0.060	m3/s

**
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$
n=0.014

Utilization 0.04 / 0.06 = **66.99** % OK (allowed for 10% for siltation)

U Channel 3 (Zone B1)

Runoff Estimation				
Design Return Period		1 in	10	years
Paved Area	121 =		121	(m2)
Unpaved Area			0	(m2)
Total Equivalent Area	121 x 0.95 + 0 x 0.35 =		115	(m2)
Time of Concentration			2.68	min
Rainfall Intensity, I *			242	mm/hr
Design Discharge Rate, Q	0.278 x 115 x 242 / 1000000 =		0.008	m3/s

$$\star \quad i = \frac{a}{(t_d + b)^c}$$

U Channel (Half round to U)			
Channel Size		300	(mm)
Gradient	1 in	450	
Area (Half Round)		0.035	(m2)
Wetted Perimeter		0.471	(m)
R		0.075	(m)
Velocity**		0.60	0.60
Capacity		0.021	m3/s

**
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$
n=0.014

Utilization 0.008 / 0.021 = **36.47** % OK (allowed for 10% for siltation)

U Channel 4 (Zone B1 + B2 + C1)

Runoff Estimation				
Design Return Period		1 in	10	years
Paved Area	760 + 511 x 0.7 =		1118	(m2)
Unpaved Area	511 x 0.3 =		153	(m2)
Total Equivalent Area	1118 x 0.95 + 153 x 0.35 =		1115	(m2)
Time of Concentration			2.68	min
Rainfall Intensity, I *			242	mm/hr
Design Discharge Rate, Q	0.278 x 1115 x 242 / 1000000 =		0.075	m3/s

$$\star \quad i = \frac{a}{(t_d + b)}$$

U Channel			
Channel Size		375	(mm)
Gradient	1 in	300	
Area		0.126	(m2)
Wetted Perimeter		0.964	(m)
R		0.130	(m)
Velocity**		1.06	m3/s
Capacity		0.133	m3/s
Utilization	0.075 / 0.133 =	56.31	%

**
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$
n=0.014

OK (allowed for 10% for siltation)

U Channel 5 (Zone [A1 + A2] + [B1 + B2 + C1])

Runoff Estimation				
Design Return Period		1 in	10	years
Paved Area	629 + 1118 =		1747	(m2)
Unpaved Area			153	(m2)
Total Equivalent Area	1747 x 0.95 + 153 x 0.35 =		1713	(m2)
Time of Concentration			2.68	min
Rainfall Intensity, I *			242	mm/hr
Design Discharge Rate, Q	0.278 x 1713 x 242 / 1000000 =		0.115	mm/hr

$$\star \quad i = \frac{a}{(t_d + b)}$$

U Channel			
Channel Size Gradient	1 in	300 90	(mm)
	1 111	0.080	(m2)
Area Wetted Perimeter		0.080	(m2) (m)
R		0.104	(m)
Velocity**		1.67	m3/s
Capacity		0.134	m3/s
Litilization	0.115 / 0.124	05 00	9/

**
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$
n=0.014

Utilization 0.115 / 0.134 = **85.88** % OK (allowed for 10% for siltation)

Time of Concentration

Catchment	Flow Distance	Highest Level		Gradient (per 100m) = (H1-H2)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + tf
Α	L			Н		
(m2)	(m)	(mPD)	(mPD)		(min)	(min)
551	34	10.1	9.8	0.882	2.68	2.68

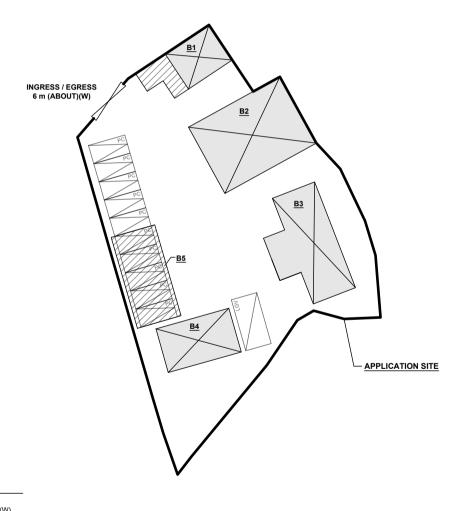
The calculated inlet time from the only external catchment (B1) is 2.68 min. $\label{eq:calculated}$

Appendix B - Proposed Development Layout Plan

	<u> </u>	<u> </u>				AREA	GFA	HEIGHT
: 1,298 n : 441 m ² : 857 m ²	1 ² (ABOUT) (ABOUT) (ABOUT)			,	RESTAURANT AND OUTSIDE SEATING ACCOMMODATION OF RESTAURANT RESTAURANT AND WASHROOM	60 m ² (ABOUT)	60 m ² (ABOUT) 40 m ² (ABOUT)	7 m (ABOUT)(2-STOREY)
: 0.45 : 34 %	(ABOUT) (ABOUT)		B2		RESTAURANT, KITCHEN & WASHROOM	, ,	138 m² (ABOUT)	3.5m (ABOUT)(1-STOREY)
: 5 : NOT AF : 589 m ²	PPLICABLE (ABOUT)		B3 (SHOP AND SERVICES OFFICE AND WASHROOM	108 m ² (ABOUT)	108 m ² (ABOUT) 108 m ² (ABOUT)	8m (ABOUT)(2-STOREY)
: 589 m²			B4		SHOP AND SERVICES	60 m ² (ABOUT)	60 m ² (ABOUT)	3m (ABOUT)(1-STOREY)
: 3 - 8 m : 1 - 2	(ABOUT)		B5		RAIN SHELTER FOR PARKING SPACE	75 m² (ABOUT)	75 m² (ABOUT)	3m (ABOUT)(1-STOREY)
					TOTAL	441 m ² (ABOUT)	589 m² (ABOUT)	



DI III DINIO





TEMPORARY SHOP SERVICES AND EATING PLACE FOR A PERIOD OF 5 YEARS

LOT 390 RP (PART) IN D.D. 106, KAM SHEUNG ROAD, YUEN LONG, NEW TERRITORIES

1:500 @ A4

LEGEND

APPLICATION SITE

PARKING SPACE

INGRESS / EGRESS

STRUCTURE (ENCLOSED)

LOADING / UNLOADING SPACE

STRUCTURE (CANOPY)

12.9.2024 MN PEVISED BY

DWG. TITLE

LAYOUT PLAN DWG NO. PLAN 4

PARKING AND LOADING/UNLOADING PROVISIONS

APPLICATION SITE AREA COVERED AREA UNCOVERED AREA PLOT RATIO SITE COVERAGE NO. OF STRUCTURE DOMESTIC GFA NON-DOMESTIC GFA TOTAL GFA BUILDING HEIGHT NO. OF STOREY

NO. OF PRIVATE CAR PARKING SPACE DIMENSION OF PARKING SPACE

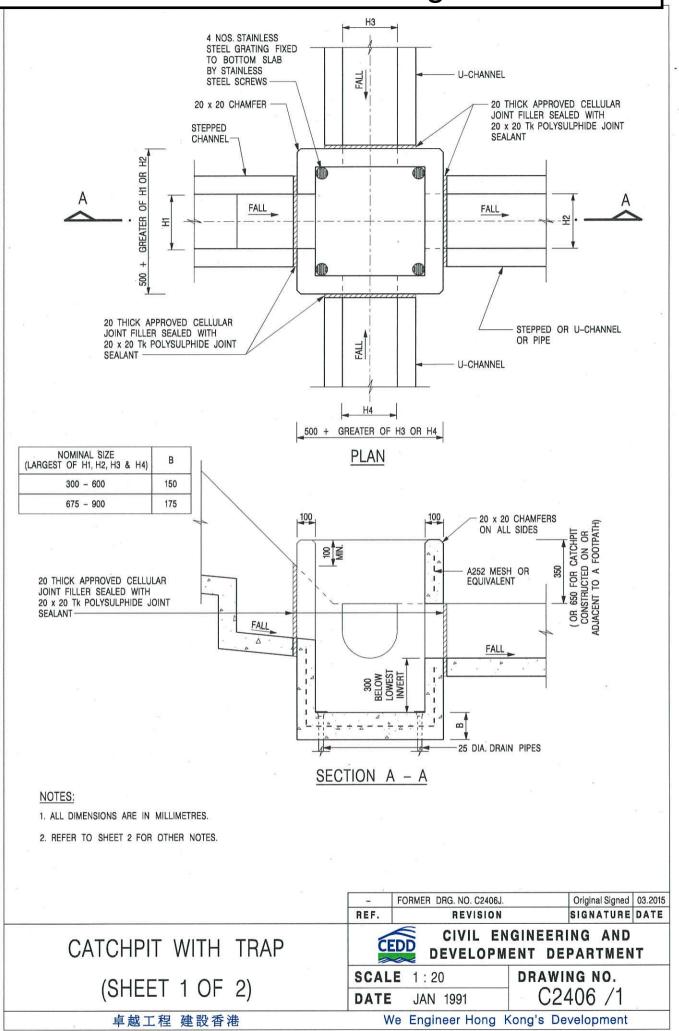
: 5m (L) X 2.5m (W)

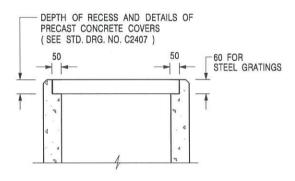
NO. OF L/UL SPACE FOR LIGHT GOODS VEHICLE

DIMENSION OF L/UL SPACE

: 7m (L) X 3.5m (W)

Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

REF.	REVISION	SIGNATURE	DATE
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
Α	MINOR AMENDMENT.	Original Signed	04.2016

CATCHPIT WITH TRAP (SHEET 2 OF 2)

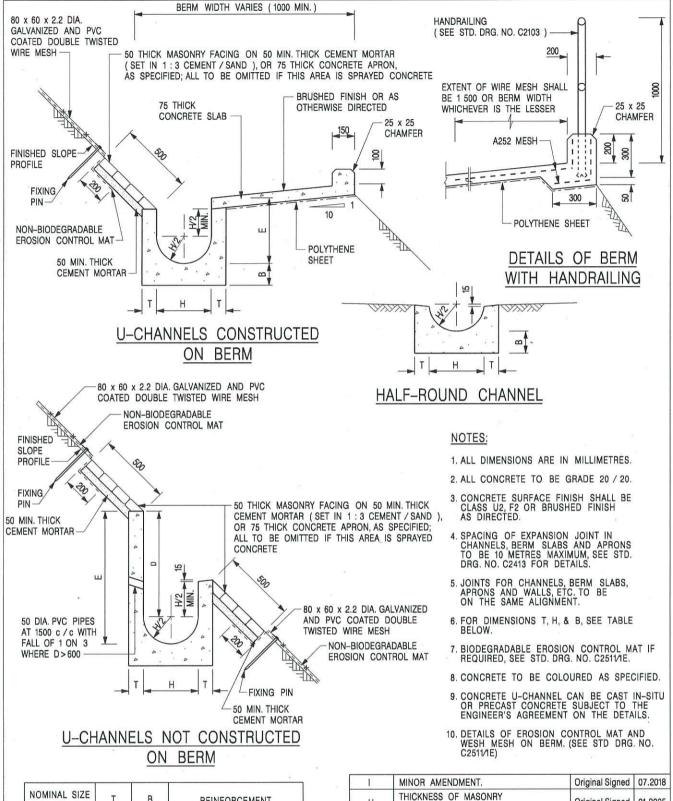


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 **DATE** JAN 1991

drawing no. C2406 /2A

卓越工程 建設香港



NOMINAL SIZE H	T	В	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100
375 - 600	100	150	WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

RE	F. REVISION	SIGNATURE	DATE
	MINOR AMENDMENTS.	Original Signed	3.94
	150 x 100 UPSTAND ADDED AT BERM	I. Original Signed	6.99
	MINOR AMENDMENT.	Original Signed	08.2001
	DRAWING TITLE AMENDED.	Original Signed	11.2001
	GENERAL REVISION.	Original Signed	12.2002
	MINOR AMENDMENT.	Original Signed	01.2004
	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
	MINOR AMENDMENT.	Original Signed	07.2018

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

卓越工程 建設香港

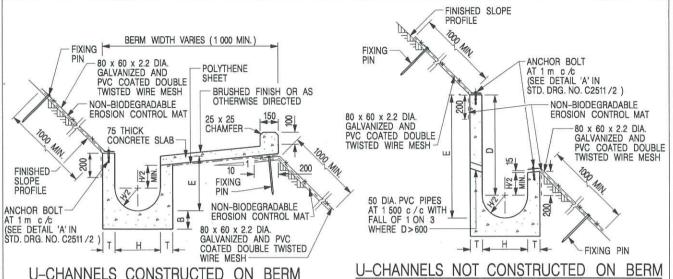
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:25

DATE JAN 1991

C2409l



U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

BIODEGRADABLE

EROSION CONTROL MAT

07.2018

12.2017

01.2005

12.2002

08 2001

6.99

3.94

10.92

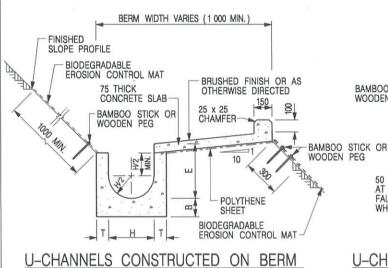
DATE

Original Signed

SIGNATURE

FINISHED SLOPE PROFILE

ш



WITH BIODEGRADABLE

EROSION CONTROL MAT

BAMBOO STICK OR WOODEN PEG

U-CHANNELS NOT CONSTRUCTED ON BERM

WITH BIODEGRADABLE

EROSION CONTROL MAT

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE TO BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- 5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- 6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- 7. FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- 8. MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- 10. THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	Ţ	В	REINFORCEMENT
300	80	100	A252 MESH PLACED
375 - 600	100	150	CENTRALLY AND T=100 WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

	DETAILS	OF	HALF-I	ROUN	D AND	
	U-CHAN	NELS	S (TYP	ЕВ-	- WITH	
I	FROSION	COL	NTROI	MAT	APRON	1)

6
CEDD
CEDD
nac

Н

G

F

E

D

C

В

A

REF.

BAMBOO STICK OR WOODEN PEG

50 DIA. PVC PIPES AT 1 500 c/c WITH FALL OF 1 ON 3

WHERE D>600

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC
DATE JAN 1991

MINOR AMENDMENT.

MINOR AMENDMENT

GENERAL REVISION.

MINOR AMENDMENT.

MINOR AMENDMENT.

MINOR AMENDMENT

FIXING DETAILS OF BIODEGRADABLE

150 x 100 UPSTAND ADDED AT BERM

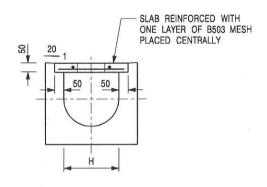
REVISION

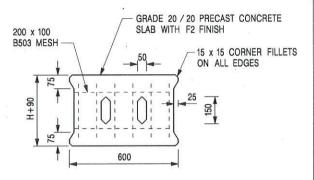
EROSION CONTROL MAT ADDED.

DIMENSION TABLE AMENDED

C2410

卓越工程 建設香港



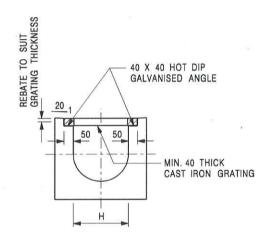


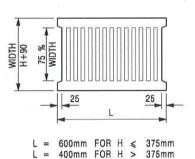
PLAN OF SLAB

TYPICAL SECTION

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)





TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

	REF.	REVISION	SIGNATURE	DATE
	Α	CAST IRON GRATING AMENDED.	Original Signed	
	В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
	С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
	D	NOTE 4 ADDED.	Original Signed	06.2008
	E	NOTES 3 & 4 AMENDED.	Original Signed	

COVER SLAB AND CAST IRON GRATING FOR CHANNELS



CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE
 1:20
 DRAWING NO.

 DATE
 JAN 1991
 C2412E

卓越工程 建設香港